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1 **The sequencing batch reactor as an excellent configuration to treat**
2 **wastewater from the petrochemical industry**

3
4 Short title: The SBR as an excellent configuration to treat wastewater from
5 the petrochemical industry
6

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19

20 **Abstract**

21 In the present study, the influence of a changing feeding pattern from continuous to
22 pulse feeding on the characteristics of activated sludge was investigated with a
23 wastewater from the petrochemical industry from the harbour of Antwerp. Continuous
24 seed sludge, adapted to the industrial wastewater, was used to start up three lab-scale
25 sequencing batch reactors. After an adaptation period from the shift to pulse feeding, the
26 effect of an increasing organic loading rate (OLR) and volume exchange ratio (VER)
27 were investigated one after another. Remarkable changes of the specific oxygen uptake
28 rate (sOUR), microscopic structure, sludge volume index (SVI), SVI₃₀/SVI₅ ratio, and
29 settling rate were observed during adaptation. sOUR increased 2 to 5 times and
30 treatment time decreased 43% in 15 days. Stabilization of the SVI occurred after a
31 period of 20 days and improved significantly from 300mL.g⁻¹ to 80mL.g⁻¹. Triplication
32 of the OLR and VER had no negative influence on sludge settling and effluent quality.
33 Adaptation time of the microorganisms to a new feeding pattern, OLR and VER was
34 relatively short and sludge characteristics related to aerobic granular sludge were
35 obtained. This study indicates significant potential of the batch activated sludge system
36 for the treatment of this industrial petrochemical wastewater.

37 **Keywords:** activated sludge adaptation; carbon removal kinetics; sequencing
38 batch reactor; settling characteristics; specific oxygen uptake rate

39 **Introduction**

40 The continuous-flow conventional activated sludge system (CAS) is the main type of
41 biological wastewater treatment system for the removal of carbon, phosphorus and
42 nitrogen from domestic and industrial wastewater. CAS has been successful for the
43 treatment of various types of wastewater, although the system is well known for sludge
44 settlement limitations due to the presence of filamentous microorganisms (Martins et al.
45 2004).

46
47 The feeding pattern was mostly investigated to prevent filamentous bulking and to
48 obtain better settling properties (Chudoba et al. 1973). In earlier research, using
49 synthetic wastewater with acetate, bactopeptone or glucose as carbon source, it was
50 demonstrated that changing the feeding pattern significantly improved the settling
51 characteristics of AS (Ciğgin et al. 2011; Cubas et al. 2011; Guo et al. 2014). All studies
52 described better settling properties when the feeding time was as short as possible (pulse
53 feeding) and the substrate gradient was as high as possible. More in detail, the research
54 of Martins et al. (2004) showed clearly the impact of the feeding time on the sludge
55 characteristics using acetate as substrate. Promoting a strong substrate gradient in the
56 SBR with an aerobic fill time ratio (AFTR) lower than 5.4% resulted in good settling
57 sludge (SVI < 120 mL.g⁻¹). Whenever acetate was added in a limiting rate with an
58 AFTR > 6.2%, a condition in which acetate concentration in the reactor was always
59 very low, the sludge settling deteriorated (SVI between 150 mL.g⁻¹ and 500 mL.g⁻¹). To
60 conclude, sludge settling could be improved by changing the feeding strategy to a pulse
61 feed in this research. Ciğgin et al. (2011) reported a SVI of almost 500 mL.g⁻¹ during
62 operation under continuous feeding. After a shift from continuous to pulse feeding, the
63 sludge settling property quickly improved to a SVI < 120 mL.g⁻¹.

64 |

65 Since the 1970s, a real revolution happened in wastewater treatment processes.
66 Advanced bubble diffusers were developed and more energy efficient material, like
67 pumps, valves and mixers were designed. Also PLC's and automation software were
68 developed. Thanks to this development, integrated real time control of the sequencing
69 batch reactor (SBR) became possible (Wiese et al. 2005; Simon et al. 2006; Shaw et al.
70 2009; Dries 2016). Despite this industrial development, the real success of the SBR
71 system remains off (Artan et al. 2001).
72 Different reasons can be formulated for the absence of a general accepted use of the
73 SBR technology. Firstly, in the authors opinion, a "lack of knowledge" is still present in
74 literature about the treatment of industrial wastewater with SBRs. Secondly inhibition
75 of sludge due to toxic components in the influent is discussed as a reason to not use
76 SBRs (De Schepper et al., 2010; USEPA, 1991). Furthermore, the CAS configuration is
77 seen as a safe operation mode in industry. Although different disadvantages, like poor
78 settling characteristics, the absence of flexible process control and low performance for
79 shock loading are known, the system is still widely used. Lastly, the role of the feeding
80 pattern for the treatment of industrial wastewaters is a topic that is still largely
81 unexplored in literature which is in contrast to the amount of studies on synthetic
82 wastewater. However, this could deliver important information about the operation
83 mode of treatment systems.
84 The objective of the current study is formulated in function of the reasons mentioned
85 above. Firstly, adaptation of AS from continuous to batch feeding for industrial
86 wastewater was investigated. After the adaptation period, the total performance of the
87 system was investigated. The sludge selected, originated from a chemical company in
88 the harbour region of Antwerp (Belgium), and was characterized by poor settling
89 properties.
90 The most important research gaps that can be filled with this research are (i) Can AS
91 adapt to a changing feeding pattern of petrochemical industrial wastewater and how
92 long does it take to adapt? (ii) How will the microbial kinetics and the physical
93 variables changes during adaptation? (iii) Can the treatment efficiency be increased by
94 changing the organic loading rate (OLR) and the volume exchange ratio (VER)?

95 **Methods**

96 ***Reactor set-up***

97 Experiments were conducted using three identical, completely automated lab-scale
98 SBRs. Respectively called SBR 1, 2 and 3. The working volume of the reactors was
99 13L with a VER of 30% to 60% and a fill time ratio lower than 1%. Every cycle, 4 to
100 8L of wastewater was added to each SBR with an internal diameter of 230 mm.

101 A Siemens (Germany) programmable logic controller (PLC), type CPU 319-3PN/DP,
102 controlled the reactor. Configuration of this PLC was done with Siemens Simatic Step7
103 software. LabView™ (National Instruments, Austin – Texas, United States of
104 America) was used as user interface to change reactor settings and to visualise the
105 process. This software was also used for data capture and data transfer to MS Excel.

106 The dissolved oxygen (DO) concentration in the reactors was monitored with a Hach
107 Lange LDO sc sensor. pH and oxidation reduction potential (ORP) were measured in

108 the reactors with Jumo TecLine sensors. Sensor signals were transferred by a process
 109 field bus (profibus) from a Hach Lange sc 1000 module to the PLC.

110 *Reactor operation*

111 The reactors were operated in three different stages, described in Table 1. At the
 112 beginning of stage I, reactors were inoculated with the industrial seed sludge. During
 113 stage I, adaptation of the sludge to a new feeding pattern was investigated. Sludge was
 114 fed once a day with 4L of wastewater. At the end of stage I, the sludge of the three
 115 reactors was mixed and divided again in the reactors. In stage II the OLR was varied
 116 between the reactors. The operation of SBR 1 was not changed, compared to stage I.
 117 The cycle time of SBR 2 and 3 was lowered to have 2 and 3 feedings a day of 4L each
 118 with a corresponding number of 2 and 3 cycles a day respectively. This resulted in a
 119 doubling and tripling of the organic loading rate for SBR 2 and 3. At the end of this
 120 period, sludge of SBR 2 and 3 was mixed and divided again in SBR 2 and 3. After the
 121 mixing, both reactors were subjected to a different VER. SBR 2 was operated with 2
 122 cycles a day with each feeding 4L of wastewater. SBR 3 was operated with 1 cycle a
 123 day with a feeding volume of 8L. Reactor 1 was not further used in stage III.

124 Table 1. Operating conditions of the lab-scale SBRs during stage I, II and III

Stage	Operation days	Reactor	Phase (duration (min))						Total cycle time (h)	Feeding volume/ cycle
			Pre-aeration	Feeding (with mixing)	Aerobic reaction	Excess sludge withdrawal	Settling	Effluent discharge		
Stage I	154	SBR1	30	2	1283	3	120	2	24	4 L
		SBR2								
		SBR3								
Stage II	220	SBR1	30	2	1283	3	120	2	24	4 L
		SBR2	15	2	639	2	60	2	12	4 L
		SBR3	10	2	425	1	40	2	8	4 L
Stage III	75	SBR2	15	2	639	2	60	2	12	4 L
		SBR3	30	4	1280	4	120	2	24	8 L

125 Dissolved oxygen concentration in the reactors was controlled between a lower and
 126 upper DO set point of 1.5 and 5.0 mg O₂.L⁻¹ respectively. In LabviewTM, data pairs of
 127 time and DO concentration were made during the air-off period to calculate the oxygen
 128 uptake rate by performing a linear least-square regression analysis.
 129

130 The pH in the reactors was kept between 7.7 and 8.8, using a sodium hydroxide solution
 131 of 3 mol.L⁻¹. The SRT was kept constant at 30 days, which is the same SRT as in the
 132 full scale industrial plant. SRT was regulated by removing a volume of 430 mL excess
 133 sludge every day. Temperature was kept at room temperature which was always
 134 between 18°C and 22°C.

135 *Sludge & influent*

136 The seed sludge and influent wastewater used in this study were obtained from a
 137 petrochemical company located in the harbour region of Antwerp. The full scale
 138 WWTP is a continuously fed AS system. The sludge retention time (SRT) of this
 139 WWTP is kept at 30 days for optimal carbon removal. The food to mass ratio (F/M) of
 140 the industrial installation is approximately $0.25 \text{ kgCOD.kgMLVSS}^{-1}.\text{d}^{-1}$.

141 During this study, thirteen different influent batches were used. The batches were taken
 142 from a buffer tank before AS treatment. Table 2 summarizes the most important
 143 parameters of the influent wastewater used in the current study. All influents were
 144 stored at 4°C to minimize biological reaction, but were given time to equilibrate to
 145 laboratory temperature before feeding to the reactors.

146
 147 Table 2. Composition of the influent batches used during stage I, II and III

Stage	Batch	day	COD ($\text{mg O}_2.\text{L}^{-1}$)	sCOD ($\text{mg O}_2.\text{L}^{-1}$)	Total nitrogen (mg N.L^{-1})	$\text{PO}_4\text{-P}$ ($\text{mg P}.\text{L}^{-1}$)	Chloride (mg.L^{-1})	Conductivity (mS.cm^{-1})	pH
I	1	1 - 39	1900	1850	10.4	0.42	3100	13.1	8.00
	2	40 - 87	1680	1640	10.6	0.29	3350	11.1	7.50
	3	88 - 144	1728	1680	5.0	0.35	3250	11.3	6.95
	4a	144 - 154	1842	1820	3.0	0.50	3500	12.0	7.08
4b	155 - 181								
II	5	182 - 212	1500	1480	0.9	0.70	2000	12.1	7.10
	6	213 - 244	1890	1820	0.5	0.50	3000	13.1	6.80
	7	245 - 275	1940	1796	0.3	1.00	3500	12.8	7.30
	8	276 - 313	1680	1625	0.6	1.10	4000	13.7	7.02
	9	314 - 343	1600	1580	0.6	0.90	3500	10.1	7.15
	10	344 - 374	1252	1180	0.4	1.20	2400	9.5	7.47
III	11	375 - 402	1504	1464	0.6	1.35	3500	12.0	7.53
	12	403 - 433	1290	1196	0.7	1.30	3550	12.2	7.82
	13	434 - 450	996	950	0.6	1.30	3550	12.4	7.41
Average and standard deviation			1600 ± 285	1544 ± 283	2.6 ± 3.7	0.8 ± 0.4	3246 ± 530	11.9 ± 1.2	7.3 ± 0.3

148 Nutrients were added to satisfy the nutritional requirements of biomass in a ratio of
 149 100:5:0.8 as COD/N/P (Eckenfelder et al. 2008). To avoid nitrification and associated
 150 oxygen consumption, nitrogen was added as KNO_3 instead of the commonly used NH_4^+ .

151 Phosphorus was added as K_2HPO_4 .

152 *Analytical methods*

153 The chemical oxygen demand (COD) was measured with Hanna Instruments (HI)
154 (Belgium, Temse) COD Tests HI 93754A-25 low range and HI 93754B-25 medium
155 range tubes. Total nitrogen was measured with the HI 94767A-50 test and chloride
156 concentration with the HI 3815 test kit, both from Hannah Instruments. pH and
157 conductivity were measured with the HI 9023 microcomputer pH meter, respectively HI
158 9033 multi range conductivity meter. Phosphorus was measured using PhosVer3
159 powder pillows from Hach Lange (Belgium, Mechelen).

160
161 Sludge volume (SV), sludge volume index (SVI), mixed liquor suspended solids
162 (MLSS) and mixed liquor volatile suspended solids (MLVSS) were measured in
163 accordance with the procedures described in Standard Methods for the Examination of
164 Water and Wastewater (1998). Settling rate, SV and SVI were determined with a 1L
165 graduated cylinder with a height of 34.5cm and an internal diameter of 6.1cm. Sludge
166 samples were always taken at the end of the aeration step and before the settling phase.
167 So sludge was always in an endogenous state.

168
169 Dissolved Organic Carbon (DOC) was analyzed during the experiments with a Sievers
170 InnovOx Laboratory Total Organic Carbon Analyzer. All samples were filtered before
171 they were analyzed using glass microfiber filters of 1.2 μ m.

172 *Image analysis*

173 Image characterization of the AS was performed with a Motic BA 310 microscope.
174 Endogenous sludge, taken before the settling phase, was used. Images were taken with
175 an EF-N Plan 10x0.25 ocular.

176 *Offline batch respirometric experiments*

177 To investigate the variation of the AS kinetics in function of time during all stages,
178 offline batch respirometric experiments were conducted. During the test, wastewater
179 was fed to endogenous excess sludge of the SBRs in a VER of 30% to compare the
180 substrate removal rate. During the test, filtered samples were taken in function of time
181 to analyse the removal of DOC. As mentioned in Kovarova-Kovar and Egli (1998), it
182 can be assumed that the concentrations of individual substrates in a complex mixture
183 decrease linearly until the substrate is exhausted. Thus, the overall DOC removal rate
184 will be the sum of the individual removal rates that are still present in the mixture.
185 Those multicomponent DOC removal kinetics were described by the Eckenfelder
186 equation (Eckenfelder et al. 2008):

$$\frac{S}{S_0} = e^{\frac{-K.X.t}{S_0}} \quad (1)$$

187 where X is the biomass concentration (mgMLVSS.L⁻¹), t is time (d), K is a first order
188 rate coefficient (d⁻¹). S₀ = start DOC substrate concentration at t₀ (mg.L⁻¹) and S = DOC
189 substrate concentration at time t (mg.L⁻¹).

190 This equation describes in a simplified way the removal of substrate (expressed as
191 DOC) in a multicomponent mixture. The above equation is experimentally proven by

192 Grau et al. (1975). K values were estimated via non-linear regression (equation 1) using
193 GraphPad software.

194 **Results and discussion**

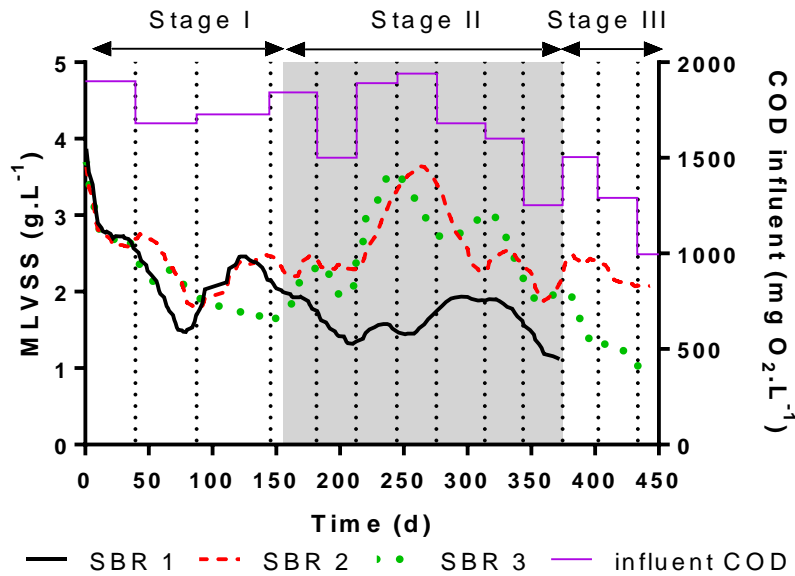
195 **Reactor performance**

196
197 The evolution of the MLVSS concentration in all reactors are shown in Figure 1. From
198 the start of stage I to the end of batch 1 (day 1 - 39) the MLVSS was 2.76 ± 0.18
199 gMLVSS.L^{-1} . After switching to a new influent batch a decrease of MLVSS in all
200 reactors was observed, which can be explained by the lower influent COD (Table 2).
201 For the 3th and 4th batch, a stabilization in the MLVSS concentration was observed.
202 For stage I, F/M ratio was $0.25 \pm 0.05 \text{ kgCOD.kgMLVSS}^{-1}.\text{d}^{-1}$ (Figure 2).
203

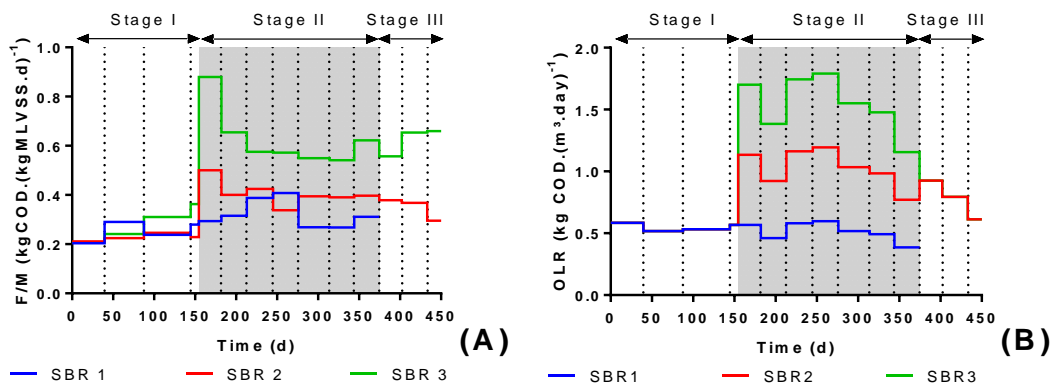
204 During stage II, different OLRs were imposed in the reactors (Figure 2 B). 7 different
205 influent batches were fed to the reactors (Table 2). Depending on the influent batch
206 used, significant changes were observed in MLVSS concentrations. Higher sludge
207 concentrations were observed in SBR 2 and 3 compared to SBR 1. This can be
208 explained by the higher OLR in SBR 2 and 3. Changing the OLR influenced the F/M
209 ratio significantly. At the beginning of stage II, F/M increased 2 and 3 times for SBR 2
210 and SBR 3 respectively, as a result of the increased OLR. Due to sludge growth in SBR
211 2 and 3, F/M decreased slowly. The average F/M during stage II was 0.32 ± 0.06
212 $\text{kgCOD.kgMLVSS}^{-1}.\text{d}^{-1}$, $0.38 \pm 0.03 \text{ kgCOD.kgMLVSS}^{-1}.\text{d}^{-1}$ and 0.57 ± 0.04
213 $\text{kgCOD.kgMLVSS}^{-1}.\text{d}^{-1}$ for SBR 1, 2 and 3 respectively.

214 In period III, different VER were applied to SBR 2 and 3. During adaptation to the new
215 VER, outspoken changes occurred in the MLVSS concentration. At the end of the
216 feeding batch 11 (day 402), SBR 2 had a MLVSS concentration twice as high as SBR 3.
217 During batch 2 and 3, the same trend was observed. The decrease of MLVSS in both
218 reactors could be explained by the lower COD of the influent. As a result of this,
219 different F/M ratios were observed. SBR 2 evolved at the end of stage III to a F/M ratio
220 of $0.29 \text{ kgCOD.kgMLVSS}^{-1}.\text{d}^{-1}$ and SBR 3 to $0.66 \text{ kgCOD.kgMLVSS}^{-1}.\text{d}^{-1}$.
221

222 During all stages a good COD effluent quality was observed. An average COD of $62 \pm$
223 $14 \text{ mgO}_2.\text{L}^{-1}$, $29 \pm 10 \text{ mgO}_2.\text{L}^{-1}$ and $31 \pm 11 \text{ mgO}_2.\text{L}^{-1}$ for stage I, II and III respectively.
224 Although wide variation of OLR and VER were imposed to the reactors, a stable
225 effluent quality was achieved which was well below Flemish discharge limits i.e. 125
226 $\text{mgO}_2.\text{L}^{-1}$.



227
 228 Figure 1: Evolution of the MLVSS in the three SBR systems and the influent COD, a
 229 dotted line represents a new feeding batch.

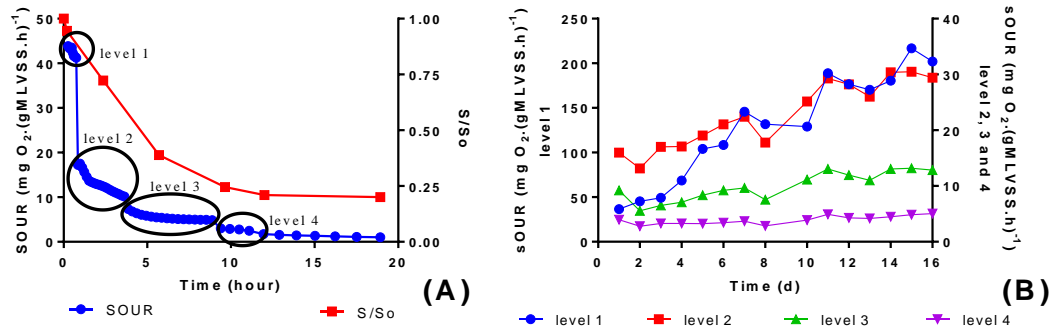


230
 231 Figure 2: Evolution of (A) the average F/M and (B) the OLR for every influent batch in
 232 each SBR. A dotted line represents a new feeding batch.

233 **Sludge activity**

234
 235 A respirogram showing the evolution of the specific OUR (sOUR) was constructed
 236 during each cycle to monitor the evolution of the sludge activity. A typical

237 (Figure 3A) consisted of 4 distinctive sOUR levels, representing four different
 238 carbon removal rates. After the fourth level, the sludge reached the endogenous
 239 state. The average sOUR values for level 1, 2, 3 and 4 in
 240 Figure 3A are respectively 43.4, 13.1, 5.6 and 2.6 $\text{mg O}_2 \cdot (\text{gMLVSS} \cdot \text{h})^{-1}$. The
 241 endogenous respiration rate was 1.6 $\text{mg O}_2 \cdot (\text{gMLVSS} \cdot \text{h})^{-1}$. The corresponding DOC
 242 removal is expressed as S/S_0 in this figure.
 243



244
245

246 Figure 3: (A) Typical specific oxygen uptake rate profile for the chemical
247 wastewater during a SBR cycle. 4 sOUR levels are identified. (B) Evolution of the
248 average sOUR for the different rate levels during the first 16 batch feeding cycles
249 for 1 SBR.

250

251 For every cycle an average sOUR value was calculated for each level defined in
252 Figure 3A.

253 Figure 3B gives an overview of the evolution of these average sOUR values during the
254 first 16 cycles. During adaptation of the continuous sludge to batch feeding, significant
255 changes in the sOUR profile were observed. Most changes were observed during the
256 first fifteen cycles, after which stabilization occurred (results not shown).

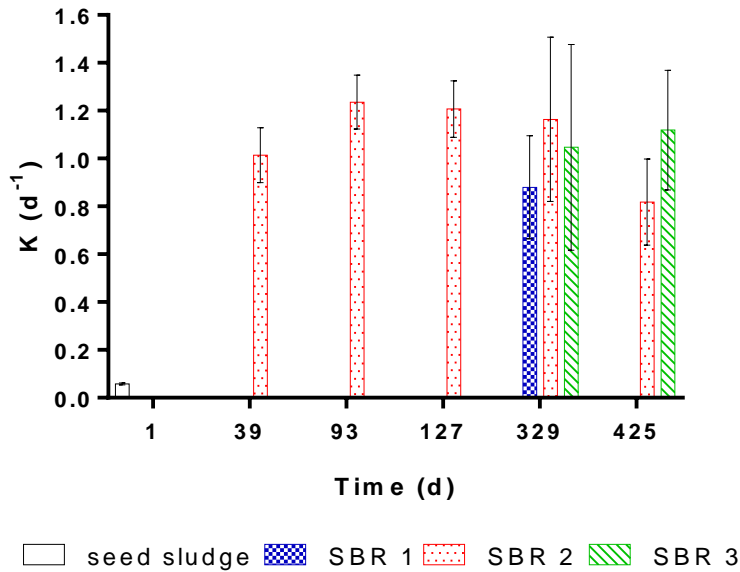
257 During the first 15 cycles, the sOUR for the most rapidly treatable components (level 1)
258 increased 5 times (36 to $190 \text{ mg O}_2 \cdot (\text{gMLVSS} \cdot \text{h})^{-1}$). For the second (level 2), third
259 (level 3) and fourth (level 4) sOUR level, sOUR increased 2 times. In combination with
260 the higher activity, the time to reach the endogenous rate decreased by 43.9% from 13.1
261 h to 7.36 h. A similar trend was observed in all 3 SBRs.

262

263 Carbon removal kinetics

264

265 DOC removal kinetics were determined during the three different stages using equation
266 1. The seed sludge had a DOC removal rate coefficient K of $0.058 \text{ d}^{-1} \pm 0.003 \text{ d}^{-1}$
267 (Figure 4). After 39 days, this value increased 17.5 times to $1.01 \text{ d}^{-1} \pm 0.11 \text{ d}^{-1}$. The
268 increased DOC removal rate corresponds to the significant increase observed in the
269 sOUR levels (Figure 3B). As shown in this research and also mentioned before in
270 literature by Grau et al. (1975) this equation can be used to describe the kinetics of a
271 multicomponent substrate removal by a mixed culture. As shown in Figure 4, DOC
272 kinetics doesn't change anymore during stage II and III. As a conclusion it can be
273 noticed that changing the OLR and VER, which leads to different F/M ratio's, did not
274 influence the removal kinetics for this wastewater.



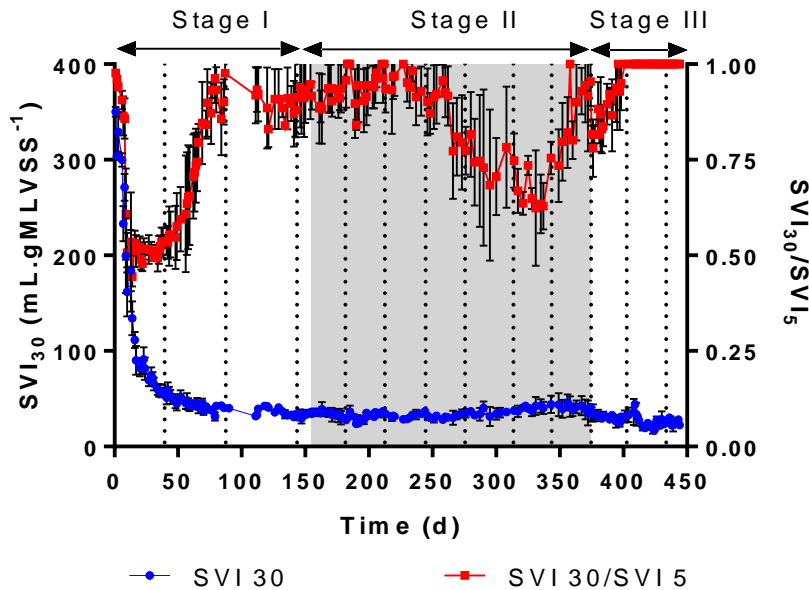
275
276 Figure 4: DOC removal kinetics for different influent batches in the 3 SBRs. Error bars
277 represent the standard error.

278 Sludge settling

279
280 Increasing the substrate gradient in the system by decreasing the length of the feeding
281 phase from continuous to pulse feeding had a strong positive impact on the sludge
282 settleability. Figure 5 shows the evolution of the average SVI, after 30 minutes settling
283 for the 3 SBRs. The most significant changes in the SVI evolution were found between
284 day 8 and 17. The SVI_{30} gradually decreased in the three SBRs from $300 \text{ mL.gMLVSS}^{-1}$
285 $^{-1}$ on day 1 to $85 \text{ mL.gMLVSS}^{-1}$ on day 20. After day 20, the SVI_{30} improved gradually
286 to a stable value of $40 \text{ mL.gMLVSS}^{-1}$ at day 120. The SVI_{30}/SVI_5 ratio evolved slowly
287 from day 20 to 100 to a value that remained stable between 0.85 and 0.95. The settling
288 rate evolved from $0,02 \text{ m.h}^{-1}$ to a rate between 10 and 13 m.h^{-1} in all SBRs
289 corresponding to a good settling sludge (Martins et al. 2004).

290
291 Throughout stage I, good settling sludge was formed. Changing the OLR at the
292 beginning of stage II had no influence on the SVI_{30} . During 220 operation days, SVI_{30}
293 stayed almost equal in all three reactors with values between 25 and $45 \text{ mL.gMLVSS}^{-1}$.
294 Changing the influent batch had no negative impact on the SVI_{30} , although between day
295 108 and 204 a less favourable SVI_{30}/SVI_5 ratio was obtained. Nevertheless, a settling
296 rate between 9.5 and 13.8 m.h^{-1} was observed in all 3 reactors during the whole stage.

297
298 The SVI_{30} of the sludge in both reactors remained unchanged after changing the VER in
299 stage III. SBR 2 and 3 had a low SVI with values between 45 and $25 \text{ mL.gMLVSS}^{-1}$.
300 After cycle 400, the SVI_{30}/SVI_5 ratio became 1. A settling rate of 12 to 13.5 m.h^{-1} was
301 measured in both SBRs.



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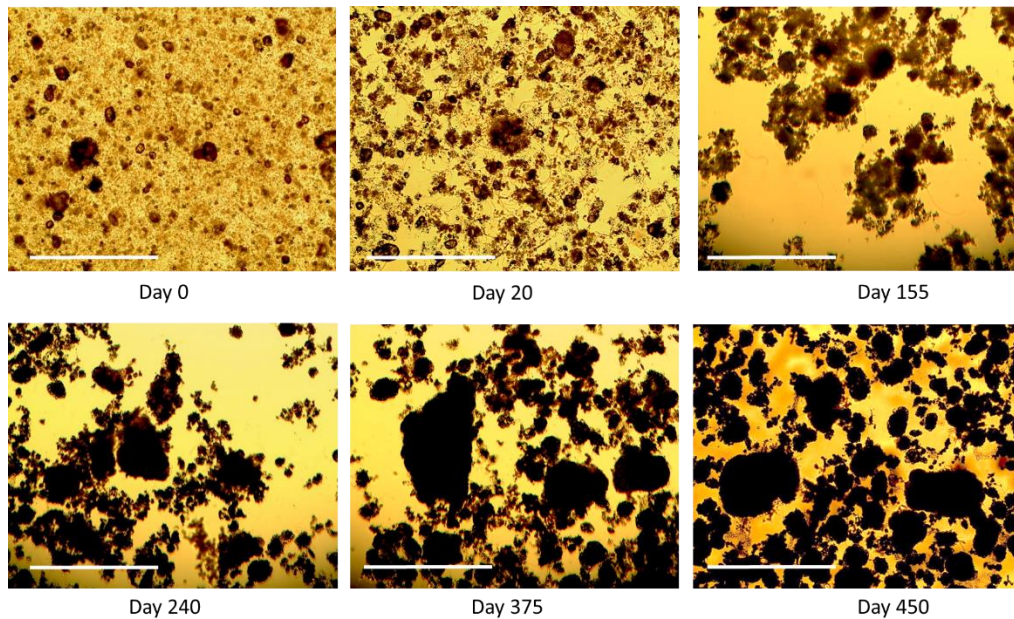
Figure 5: Evolution of the average SVI₃₀ and SVI₃₀/SVI₅ in the three SBR systems, a dotted line represents a new feeding batch. Error bars represent the standard deviation between the three SBRs.

307 Sludge morphology

308 Figure 6 illustrates the morphological changes of the sludge. The first picture in Figure
309 6 shows the seed sludge from the industrial WWTP. The sludge had a loose and
310 irregular structure. After 10 days of SBR operation, more compact flocs appeared. After
311 20 days dark spots were formed. Between day 20 and 80, the formed flocs became
312 larger and darker. After 80 days robust and stable flocs were formed. This
313 morphological adaptation was observed in all three SBRs.

314 At the beginning of stage II, flocs with a diameter of 100µm were observed in the
315 sludge. During the transition to a new OLR, flocs became bigger in all reactors. At the
316 end of stage II, all reactors had a good settleable sludge (Figure 5). This corresponds to
317 the well-defined sludge flocs that were observed during the microscopic investigation.
318

319 During stage III, the microscopic structure of the sludge in SBR 2 and 3 underwent no
320 significant changes, which is in correlation to the sludge settling characteristics. Well-
321 shaped flocs were preserved. In accordance to granular sludge, granule size has to be at
322 least 0.2mm (de Kreuk et al. 2007). Although the floc size of the granules was not
323 measured it can be observed that a substantial amount of the flocs had a size bigger than
324 0.2mm.
325



326
 327 Figure 6: Evolution of the sludge morphology (40x magnified) during the whole
 328 experiment. The white scale bar represent a length of 500 μ m.

329 Discussion

330
 331 In this study the effect of a changing feeding pattern, from continuous to batch/pulse, on
 332 the characteristics of AS was studied. Van den Broeck (2009) mentioned a significantly
 333 different transition period for different sludge characteristics (SVI, sOUR, microscopic
 334 structure) which was also confirmed in this study. Changing the feeding pattern resulted
 335 in rapid and significant improvement of the microbial kinetics expressed as sOUR. The
 336 adaptation time corresponded to half a sludge age. This is significantly faster than
 337 reported in Van den Broeck et al. (2009) who showed an adaptation of approximately
 338 20 days for the sOUR with a sludge age of 20 days after changing the feeding pattern
 339 and influent composition of domestic AS. Earlier research from Chudoba et al. (1973,
 340 1985), Martins et al. (2003), Van den Eynde et al. (1983) and Verachtert et al. (1980)
 341 using synthetic wastewater and Van den Eynde et al (1982) using industrial wastewater
 342 already stressed the importance of the feeding pattern for the settling characteristics of
 343 the sludge which is also confirmed in this study with a real petrochemical industrial
 344 chemical wastewater.

345
 346 A stable and robust AS system was formed during the first adaptation period. Changing
 347 the VER, OLR and influent composition had no significant influence on the settleability
 348 of the sludge. During the whole period, SVI values between 25 and 45 mL.gMLVSS⁻¹
 349 were achieved. Carbon removal rate kinetics showed no significant difference between
 350 the reactors, but there was a significant improvement during adaptation. In accordance
 351 to the F/M ratio, a higher ratio was obtained with the highest VER of 60% and F/M
 352 ratios 2 to 3 times higher than in the industrial installation. The bad settling sludge was
 353 turned over into a good settling sludge in 20 days which was also achieved in Martins et
 354 al. (2004) Martins showed an adaptation time of 7 days for the SVI after changing the
 355 feeding pattern from continuous to pulse feeding with a SRT of 10 days. This is 0.7
 356 times the sludge age which is almost the same as for this wastewater with a value of
 357 0.67. Since the industrial company struggles with bad settling sludge (Figure 5),
 358 changing the feeding pattern could mean a beneficial improvement for the settling. The

359 obtained settling properties in the present study correlated to the characteristics of
360 granular sludge. As defined in de Kreuk et al. (2007), granular sludge has a settling rate
361 higher than $10\text{m}\cdot\text{h}^{-1}$ and a $\text{SVI}_{30}/\text{SVI}_5$ equal to 1 which is also obtained in this study.
362 The observed effects can, to a large extent, be explained by the kinetic selection theory
363 of Chudoba et al. (1973, 1985), who have shown that the activity of microorganisms
364 depends on substrate composition and concentration. This implies that (among many
365 other factors) the feeding pattern in the reactor exerts an important influence on
366 microbial dynamics in the AS.

367

368 Growing interest last years is observed for the SBR technology to treat wastewater since
369 some important benefits can be addressed to the SBR (Mace and Mata-Alvarez 2002):
370 (i) lower cost than conventional biological treatment methods (ii) lower footprint (iii)
371 flexible for wide swings in hydraulic and organic loadings (iv) easier to control
372 filaments and settling problems and (v) dynamic process control.

373 Nevertheless, the number of SBR reactors is still low in industry, especially in the
374 (petro)chemical industry. In the authors opinion, a lack of information is still present in
375 literature about SBR treatment of this type of wastewater. Mostly fear for sludge
376 inhibition is referred to as a reason to choose for other systems than SBR (USEPA
377 2000). Interestingly, different researchers stated the benefits of the SBRs, compared to
378 the CAS system, to remove difficult degradable (toxic) components. For example,
379 Papadimitrou (Papadimitriou et al., 2009) compared the adaptation of activated sludge
380 for wastewater that contained phenol and cyanide. They clearly showed the superior
381 removal capacity of the SBR system to remove both pollutants. Furthermore, the
382 authors showed better performance to shock loading of these chemicals in the SBR and
383 inactivation and disintegration of the sludge with phenol concentrations above 800
384 $\text{mg}\cdot\text{L}^{-1}$ was seen in the CAS system. In addition, the study clearly showed better settling
385 properties in the SBR. Moreno and Buitrón (2012) (Moreno-Andrade and Buitrón,
386 2012) investigated the degradation of 4-methylaniline in a SBR. They demonstrated the
387 ability of AS to adapt to this difficult degradable chemical. In (Moreno-Andrade and
388 Buitrón, 2004) the capability of the SBR is presented to degrade 4-chlorophenol. In the
389 present study it is shown that inhibition of activated sludge was not relevant for the
390 petrochemical wastewater used. The activated sludge showed a good performance even
391 when subjected to triplication of the OLR and doubling of the VER.

392

393 To conclude, the researchers have demonstrated that activated sludge from a CAS
394 system could quick and easily adapt to pulse fed activated sludge in a SBR reactor.
395 Furthermore, activated sludge characteristics evolved positively. In this research, lab
396 scale SBR results were compared with the full scale results. Future work has to be done
397 to compare the lab scale SBR results with a lab scale CAS system to exclude effects like
398 different degrees of mixing in the full scale plant, substrate gradients in the perfectly
399 mixed processes, differences in pH or nutrients addition and variable influent
400 composition of the wastewater or different organic load rate or oxygen concentration in
401 the reactor.

402

Conclusions

403 The results of the experiments allow the authors to conclude that batch feeding,
404 significantly influenced the characteristics of the microbial population of the AS in a
405 positive way. In SBR systems more advantageous treatment rates and improved settling
406 characteristics can be achieved in comparison to continuous flow systems with the

407 chemical wastewater. As a result of this higher activity due to changing the feeding
408 pattern, the industrial treatment plant could be built more compact and higher
409 wastewater flows could be treated.
410

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